

Dimensionnement des chaussées souples méthode mécaniste- empirique

Alan Carter
ÉTS

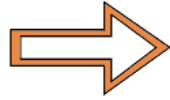


Plan de la présentation

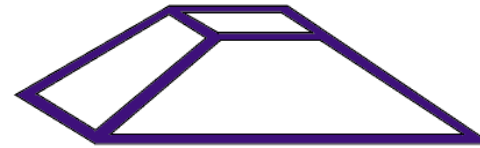
- Qu'est qu'une méthode M-E
 - Définition
 - Justification
- MEPDG
 - Historique
 - Principaux points à considérer
- Exemple avec nos données
- Conclusion



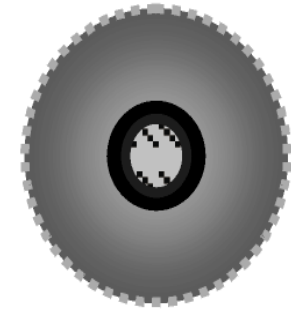
Climat



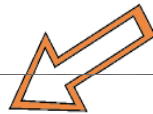
Matériaux



Structure

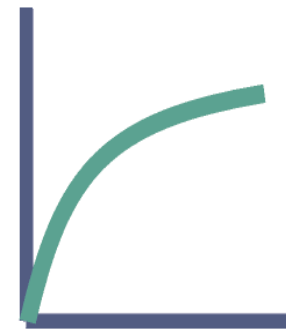
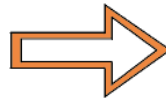


Charge



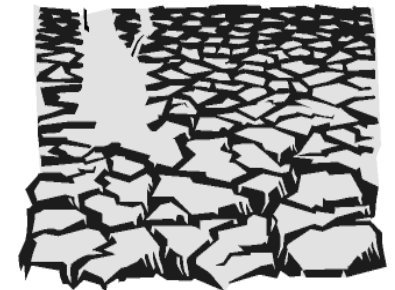
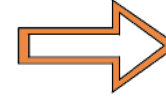
Réaction

Dommages



Période de temps

**Accumulation
de dommages**



Désordres

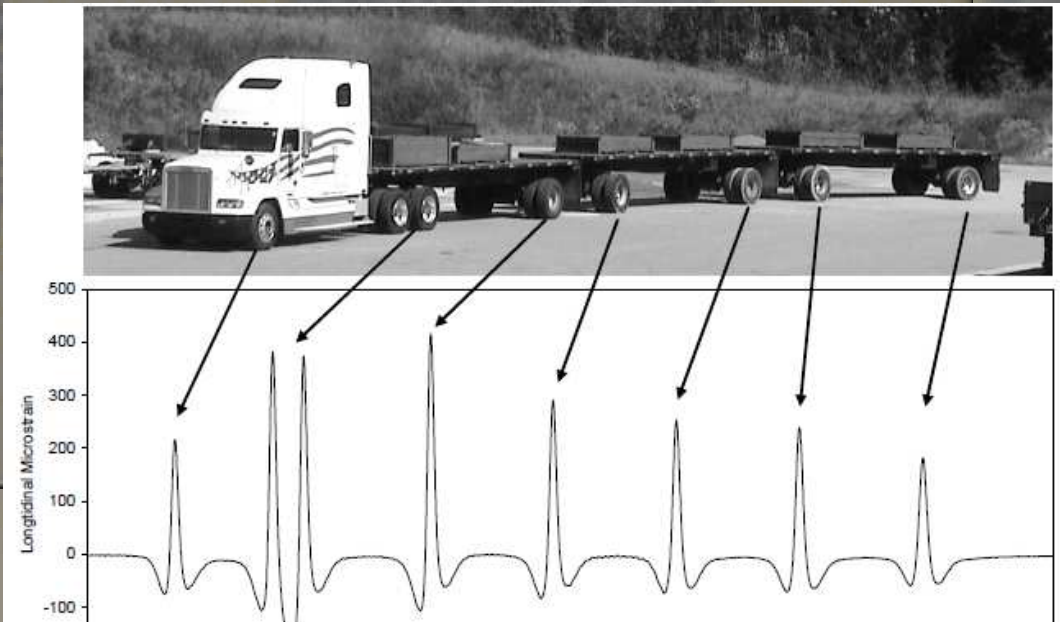
M-E?

- Méthode mécanistique-empirique

- Partie mécanique

- Calcul les contraintes et déformations dans un système multi-couche soumis à des charges extérieures, aux effets de la températures et de l'humidité

- Modélisation du comportement des matériaux inclut dans la chaussée

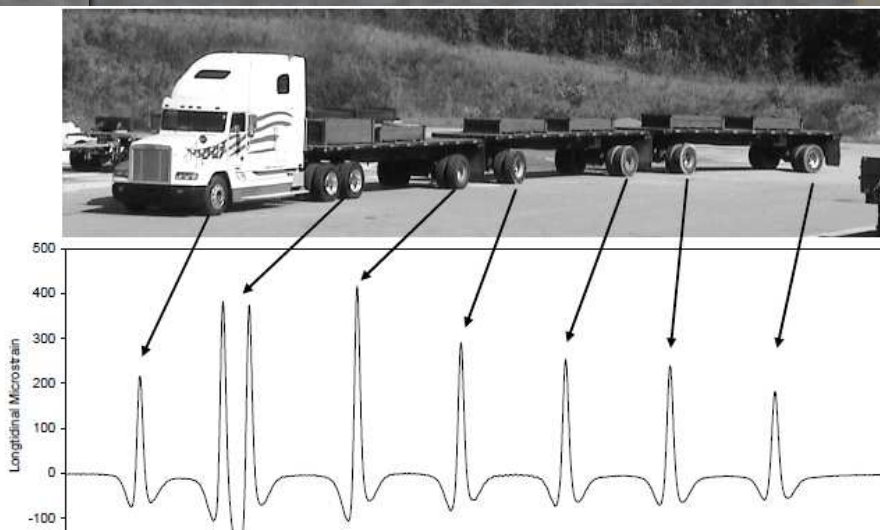


M-E?

- Méthode mécanistique-empirique

- Partie empirique

- Lien entre les caractéristiques mécaniques et les dégradations de la chaussée
 - corrélations



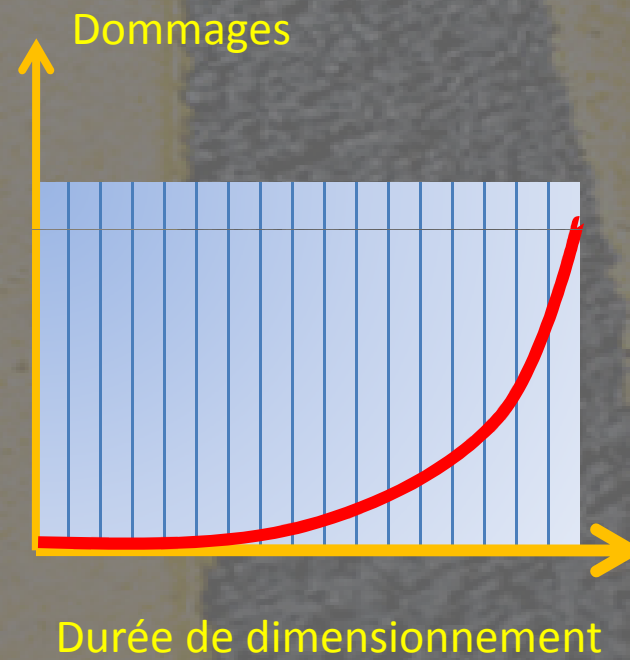
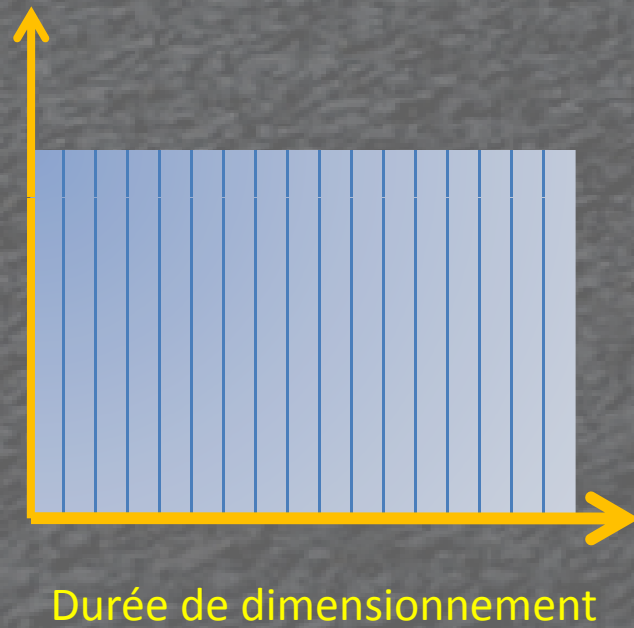
?

=



M-E?

● principe



M-E?

- Hypothèse de Miner

$$\sum \frac{n}{N} = D$$

Pourquoi pas juste mécanique?

- Manque de connaissances
- Manque de données
- Pas nécessairement utile...



Historique

- Chaussées rigides

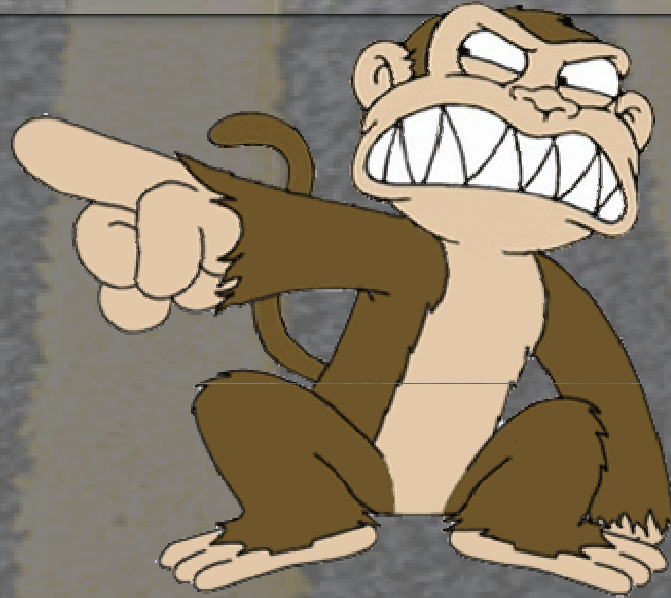
- 1938 Bradbury
- Dalle structurale
- Travaux toujours en cours

- Chaussées souples

- 1940 Burminster
- Hypothèse
 - La chaussée peut être modélisé comme une structure multi-couche élastique ou visco-élastique sur une fondation élastique ou visco-élastique
- Premier développement méthode mécaniste: Ivanov (URSS) et McLeod (Canada)

M-E utilisable

- Chaussées rigides
 - Méthode PCA
- Chaussées souples
 - Kentucky DOT
 - Asphalt Institute
 - Shell International
 - Minnesota DOT



Pourquoi une méthode M-E?

- Nos chaussées sont en bons états...



Pourquoi une méthode M-E?

● Trafic

● Augmentation du nombre

- De 5 à 15×10^6 en 1960 à 50 à 200×10^6 aujourd'hui

● Augmentation de la période de conception

- 20 ans en 1960 à 30-40 ans maintenant

● Changement de la configurations des essieux

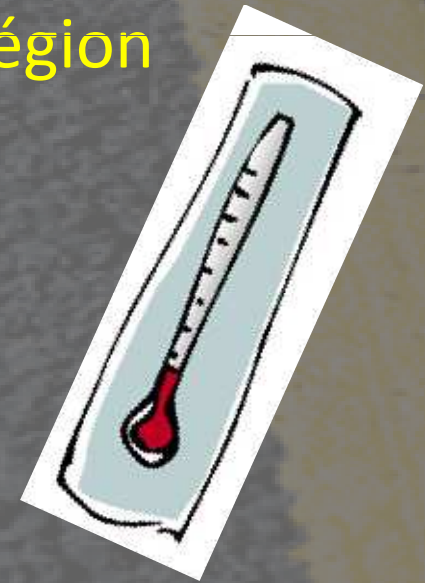
● Pression des pneus

- 80 vs 120 psi



Pourquoi une méthode M-E?

- Environnement
 - Méthode AASHTO basé sur le test track
 - Basé sur climat local
 - Utilisation d'une moyenne dans la région



Pourquoi une méthode M-E?

- Matériaux

- Surface

- Modèle non complet

- Base

- Modèle sur 2 types de base seulement

- Infra

- Caractérisation incomplète

Pourquoi une méthode M-E?

- Lien avec performance
 - Méthode empirique basé sur PSI
 - Besoin d'une méthode qui prédit la fissuration par fatigue, fissuration thermique, orniérage, écaillage, arrachement, etc...

Problèmes avec méthode M-E

- Énormément de facteur à prendre en considération
- Nécessite un ordinateur
- Besoin de données pas nécessairement disponible!



Méthode M-E AASHTO

- Commencé en 1996
- AASHTO abandonne
- Devient MEPDG
 - **Mechanical Empirical Pavement Design Guide**
- Chaussées souples et rigides
- 2009 groupe de travail pour DARWIN-ME



NCHRP

M-E PDG

Mechanistic-Empirical Pavement Design Guide

This software is for review only and should not be used for design.
This software was developed under NCHRP 1-37A and 1-40D.
Distribution of this software must be approved by NCHRP.

developed by



APPLIED RESEARCH ASSOCIATES, INC

TRANSPORTATION

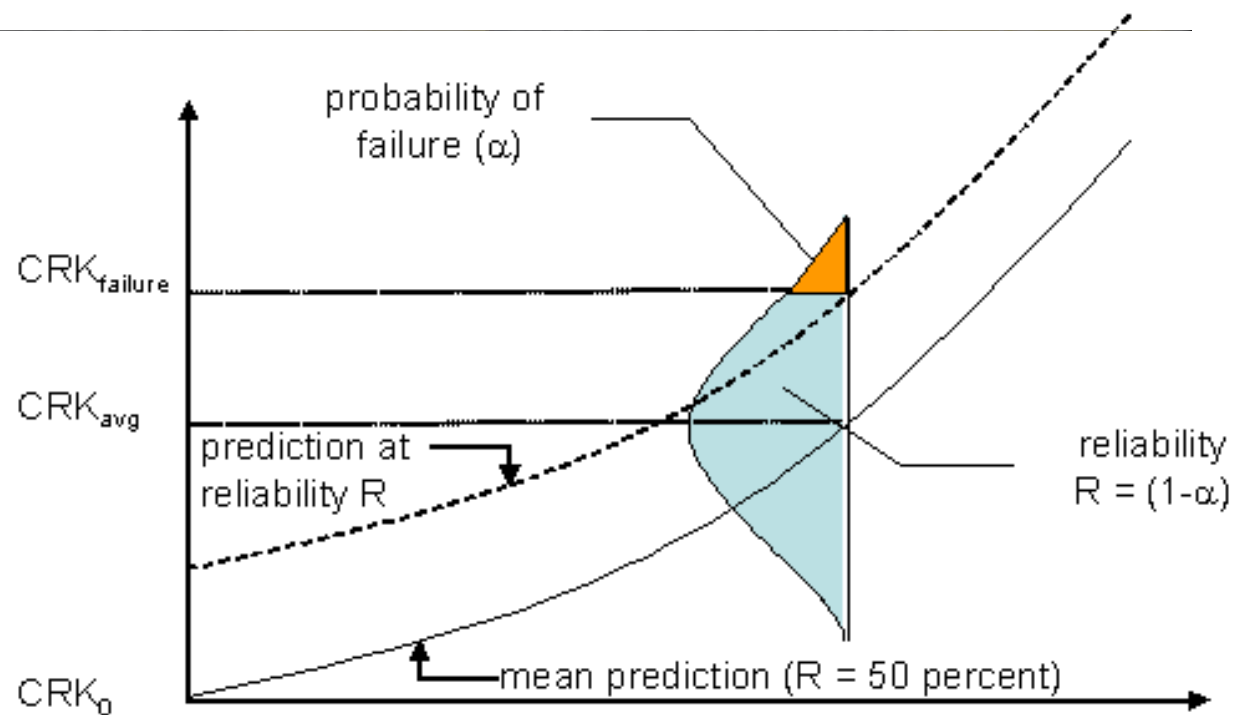


Critères de conception

	Limit	Reliability
☒ Terminal IRI (in/mile)	482	90
☒ AC Surface Down Cracking Long. Cracking (ft/mi)	2000	90
☒ AC Bottom Up Cracking Alligator Cracking (%)	25	90
☒ AC Thermal Fracture (ft/mi)	1000	90
☒ Chemically Stabilized Layer Fatigue Fracture(%)	25	90
☒ Permanent Deformation - Total Pavement (in)	0.75	90
☒ Permanent Deformation - AC Only (in)	0.25	90

Confiance - Reliability

- Probabilité qu'une structure de chaussée donnée «survive» le temps prévue lors de la conception



MEPDG

● Étapes de conceptions

1. Le concepteur entre une conception initiale
2. Le logiciel estime le damage et les principales dégradations durant la durée de vie
3. La conception est vérifiée en fonction des critères de performances et est modifiée au besoin

Les intrants

- Trafic
- Environnement
- structure

Trafic

- DJMA-camion
 - Dans les deux directions
 - Classe 4 et plus

Class 4



Class 5



Class 6



Class 7



Class 8



Class 9



Class 10



Class 11



Class 12



Class 13



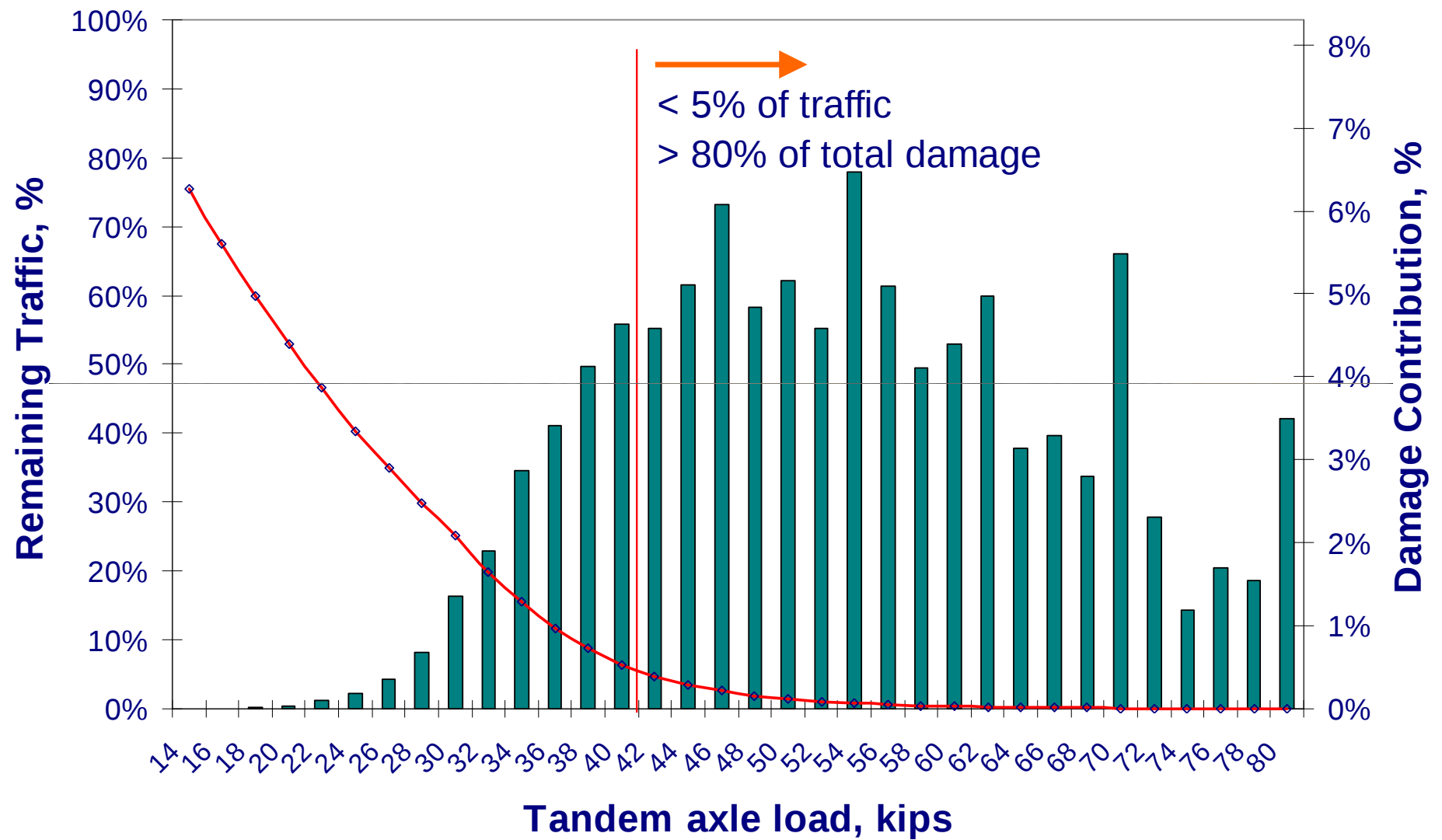


Table 2: FHWA Vehicle Classification (from FHWA, 2001)¹

Class	Type	Description	Typical ESALs per Vehicle²
1	Motorcycles	All two- or three-wheeled motorized vehicles. Typical vehicles in this category have saddle type seats and are steered by handle bars rather than wheels. This category includes motorcycles, motor scooters, mopeds, motor-powered bicycles, and three-wheel motorcycles. This vehicle type may be reported at the option of the State.	negligible
2	Passenger Cars	All sedans, coupes, and station wagons manufactured primarily for the purpose of carrying passengers and including those passenger cars pulling recreational or other light trailers.	negligible
3	Other Two-Axle, Four-Tire Single Unit Vehicles	All two-axle, four tire, vehicles, other than passenger cars. Included in this classification are pickups, panels, vans, and other vehicles such as campers, motor homes, ambulances, hearses, and carryalls. Other two-axle, four-tire single unit vehicles pulling recreational or other light trailers are included in this classification.	negligible

4	Buses	All vehicles manufactured as traditional passenger-carrying buses with two axles and six tires or three or more axles. This category includes only traditional buses (including school buses) functioning as passenger-carrying vehicles. All two-axle, four-tire single unit vehicles. Modified buses should be considered to be a truck and be appropriately classified.	0.57
5	Two-Axle, Six-Tire, Single Unit Trucks	All vehicles on a single frame including trucks, camping and recreational vehicles, motor homes, etc., having two axles and dual rear wheels.	0.26
6	Three-Axle Single Unit Trucks	All vehicles on a single frame including trucks, camping and recreational vehicles, motor homes, etc., having three axles.	0.42
7	Four or More Axle Single Unit Trucks	All trucks on a single frame with four or more axles.	0.42
8	Four or Less Axle Single Trailer Trucks	All vehicles with four or less axles consisting of two units, one of which is a tractor or straight truck power unit.	0.30
9	Five-Axle Single Trailer Trucks	All five-axle vehicles consisting of two units, one of which is a tractor or straight truck power unit.	1.20
10	Six or More Axle Single Trailer Trucks	All vehicles with six or more axles consisting of two units, one of which is a tractor or straight truck power unit.	0.93
11	Five or Less Axle Multi-Trailer Trucks	All vehicles with five or less axles consisting of three or more units, one of which is a tractor or straight truck power unit.	0.82
12	Six-Axle Multi-Trailer Trucks	All six-axle vehicles consisting of three or more units, one of which is a tractor or straight truck power unit.	1.06
13	Seven or More Axle Multi-Trailer Trucks	All vehicles with seven or more axles consisting of three or more units, one of which is a tractor or straight truck power unit.	1.39

Trafic

- Ajustement

- Mensuel

- Trafic annuel ajusté à chaque mois

- Classe de véhicule

- Ajustement en fonction des classes

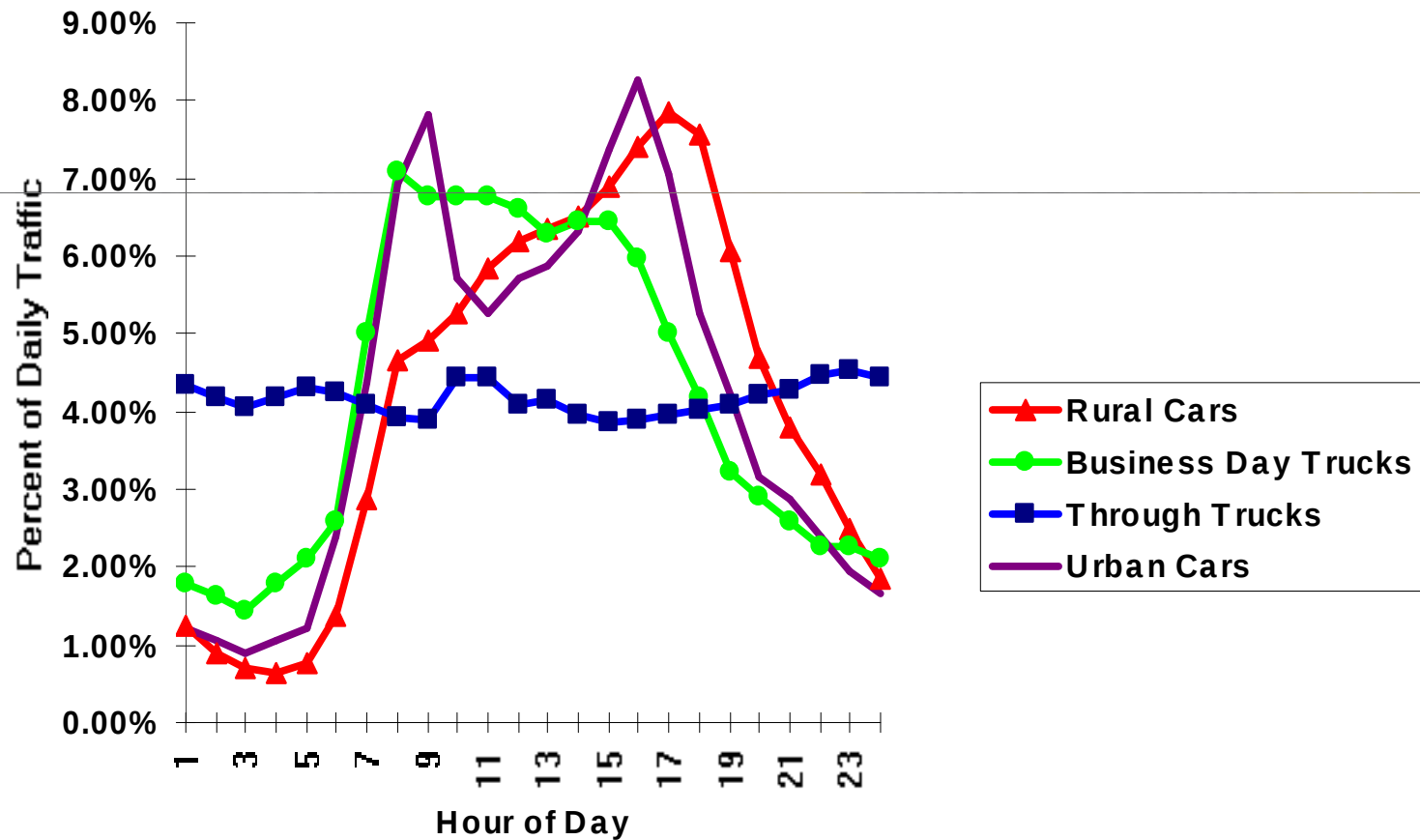
- Horaire

- Trafic annuel ajusté à chaque heure

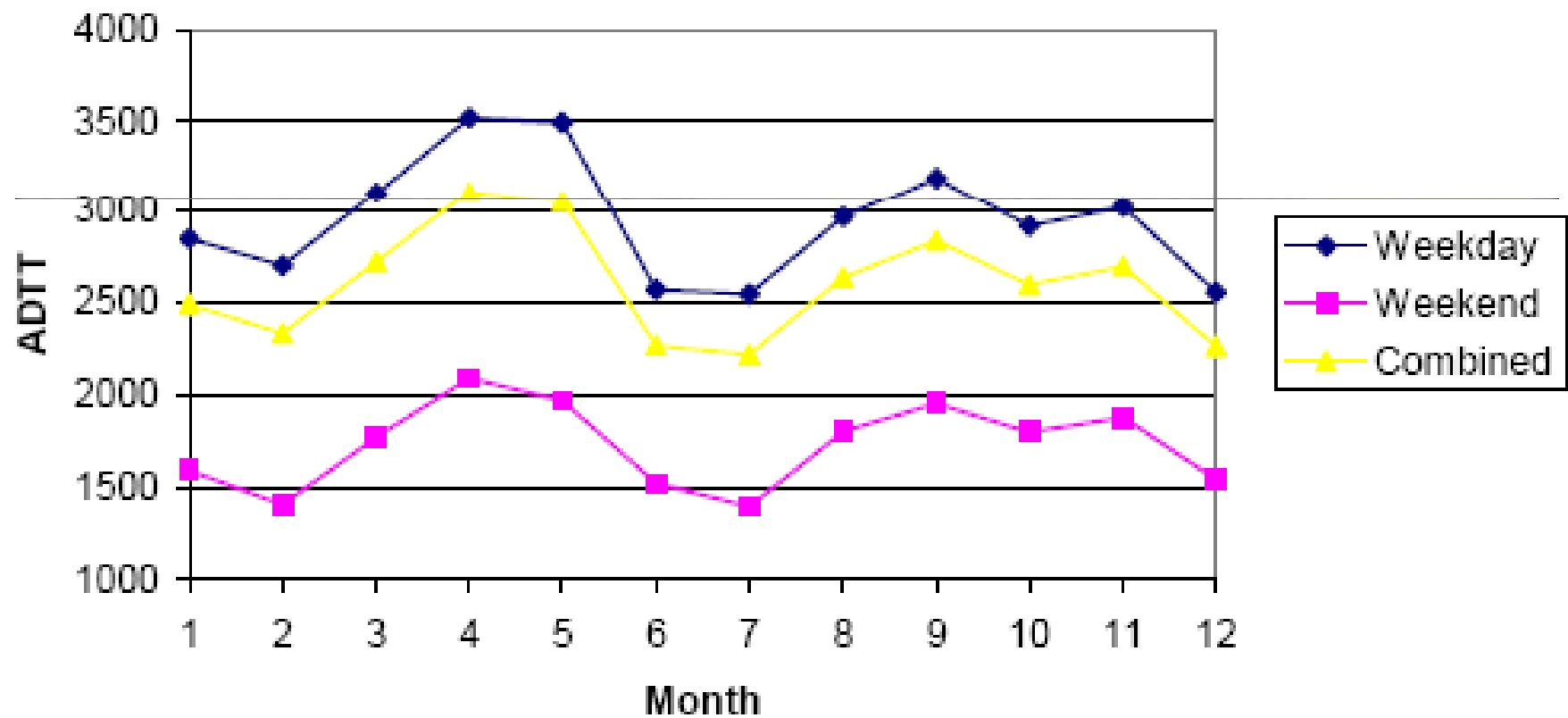
- Accroissement du trafic

- Pas d'accroissement, linéaire, composé

Traffic



Traffic



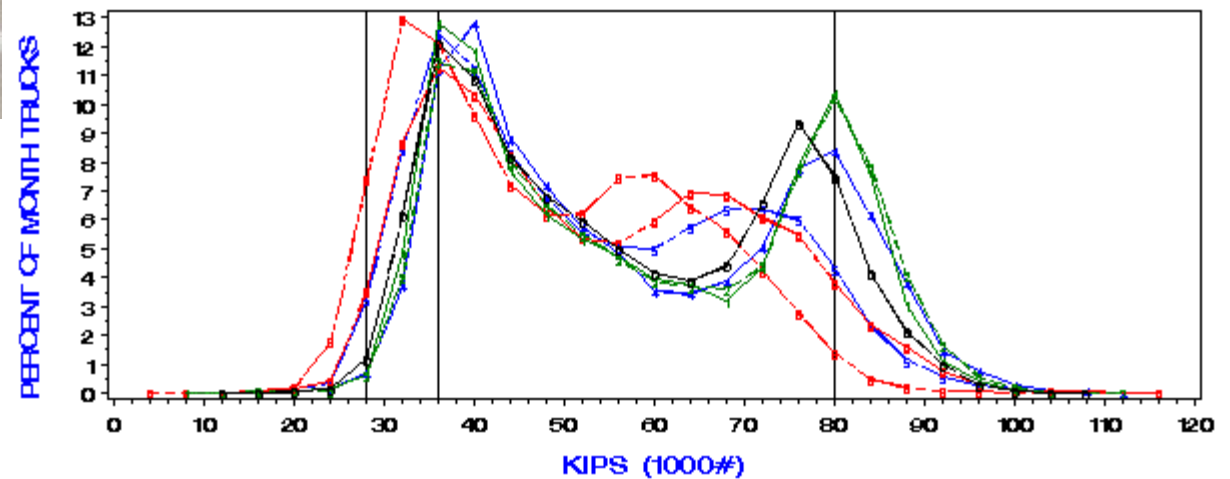
Charge sur essieux

- Doit définir charge sur chaque essieux pour chaque catégorie de véhicules à chaque mois
- Calculer avec WIM

WIM



DISTRIBUTION OF GROSS WEIGHT 5-AXLE SEMIS BY STATION DIRECTION STA= P3S NAME= 185TH_ DIR= 5 LANE= 2 YY= 2002

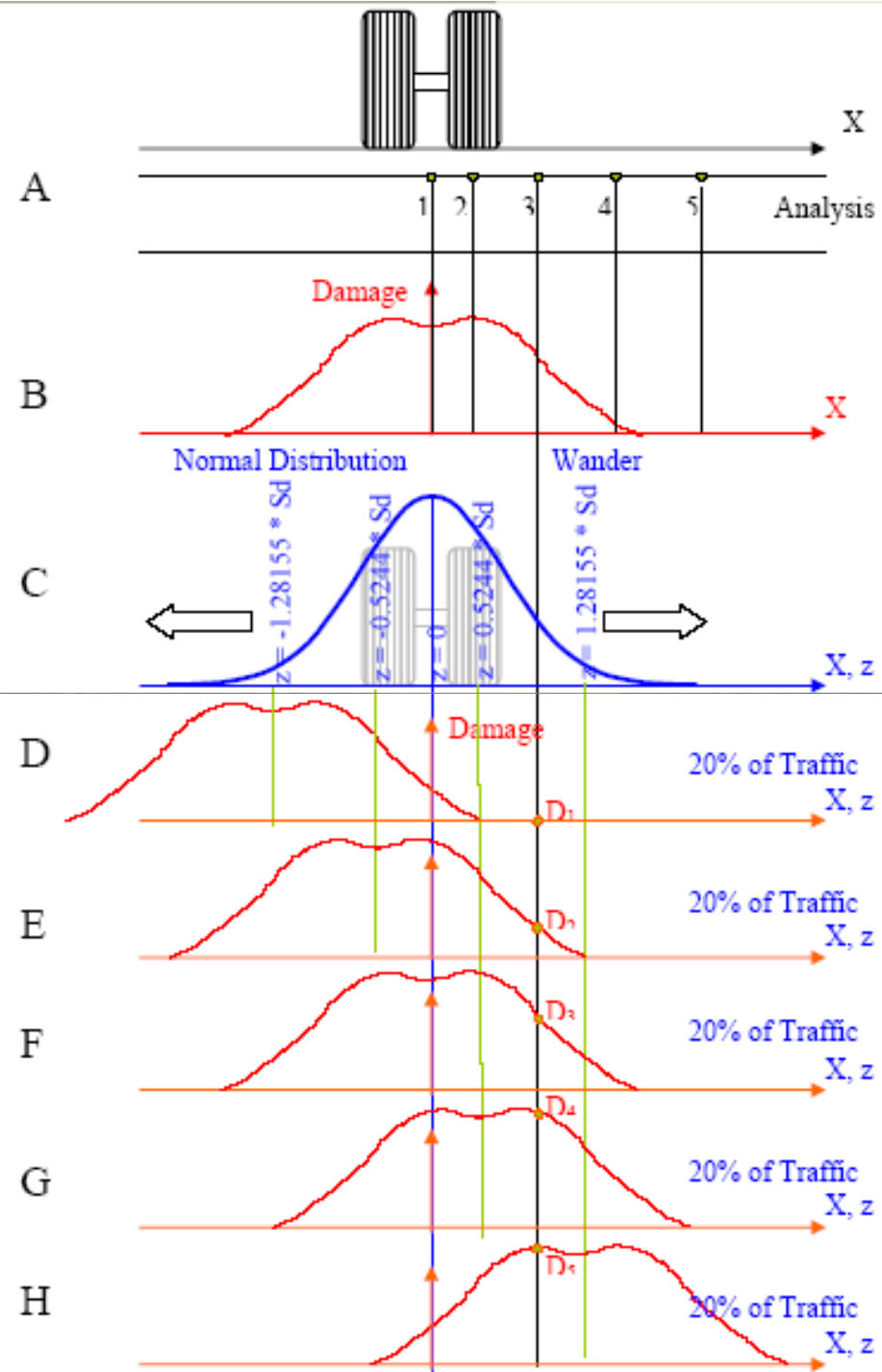


LEGEND: 1-3= JAN - MAR 4-6= APR - JUN 7-9= JUL - SEP O,N,D= OCT,NOV,DEC

WAPROD PROG: WGRAPH.MN DATA: MONTH FILES
FILE: O:\JAN-DEC\OARD.P3S\ST02.P3S FROM CATALOG: G:\WIM\OFFICE\GRAPH\WV\ST02.P3S

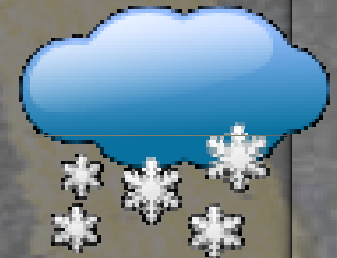
Trafic

- Configurations des essieux
- Position des charges sur la chaussée



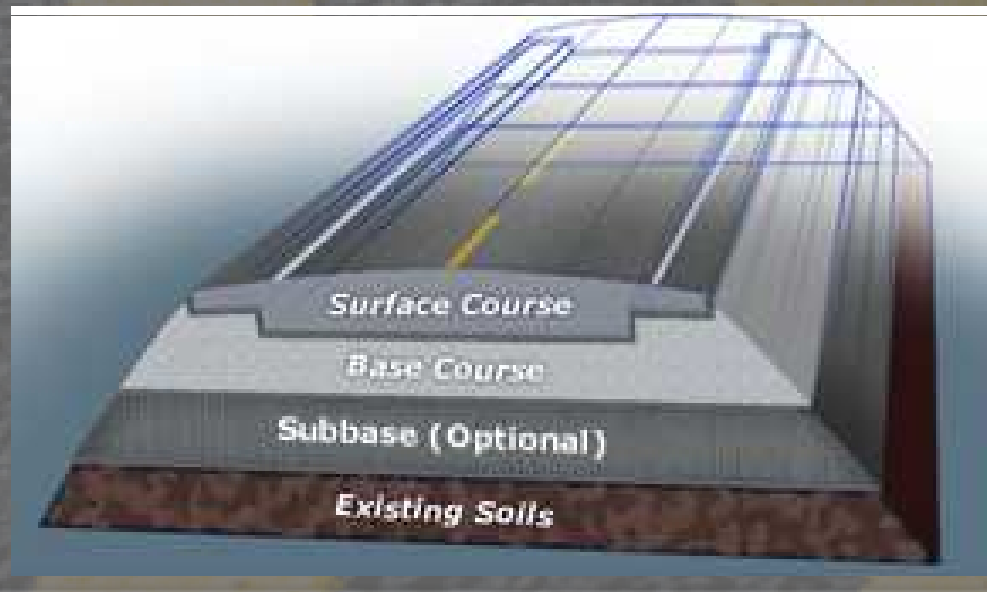
Climat

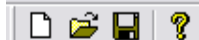
- Température
- Humidité
- Profondeur de la nappe phréatique



Structure

- Utilisation de modèle mathématique pour
 - Évaluation des contraintes
 - Évaluation des modules
 - ...





Project [C:\DG2002\Projects\Project1.dgp]

- General Information
- Site/Project Identification
- Analysis Parameters

Inputs

- Traffic
 - Traffic Volume Adjustment Factors
 - Monthly Adjustment
 - Vehicle Class Distribution
 - Hourly Truck Distribution
 - Traffic Growth Factor
 - Axle Load Distribution Factors
 - General Traffic Inputs
 - Number Axles/Truck
 - Axle Configuration
 - Wheelbase
- Climate
- Structure
- Layers

Results

- Input Summary
 - Project
 - Traffic
 - Climatic
 - Design
 - Layer
- Output Summary

Analysis Status:

Analysis	% Complete
----------	------------

General Project Information:

Parameter	Value
Type	New Flexible
Design Life	20 Years
Climate	
Construction Date	9/2006
Traffic Open Date	10/2006
Initial AADTT	

Properties

Setting	Value
Units	US Customary
Analysis Type	Probabilistic
Output Type	Excel Worksheet
Warnings	Enabled

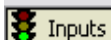


Run Analysis



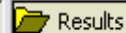
Project [C:\DG2002\Projects\BQ_cas_2_50ans_c.dgp]

- General Information
- Site/Project Identification
- Analysis Parameters



Inputs

- Traffic
 - Traffic Volume Adjustment Factors
 - Monthly Adjustment
 - Vehicle Class Distribution
 - Hourly Truck Distribution
 - Traffic Growth Factor
 - Axle Load Distribution Factors
 - General Traffic Inputs
 - Number Axles/Truck
 - Axle Configuration
 - Wheelbase
- Climate
- Structure
 - HMA Design Properties
 - Layers
 - Layer 1 - Asphalt concrete
 - Layer 2 - Asphalt concrete
 - Layer 3 - Crushed stone
 - Layer 4 - Crushed stone
 - Layer 5 - SM
 - Thermal Cracking



Results

- Input Summary
 - Project
 - Traffic
 - Climatic
 - Design
 - Layer
- Output Summary
- Flexible Summary
 - Layer Modulus
 - AC Modulus (plot)
 - Fatigue Cracking
 - Surface Down Damage (plot)
 - Surface Down Cracking (plot)
 - Bottom Up Damage (plot)
 - Bottom Up Cracking (plot)
 - Thermal Cracking
 - Crack Depth (plot)
 - Thermal (C-h) (plot)
 - Crack Length (plot)
 - Crack Spacing (plot)
 - Rutting
 - Rutting (plot)
 - IRI (plot)

Analysis Status:

Analysis	% Complete
Traffic	100%
Climatic	100%
Thermal Cracking	100%
AC Analysis	100%
Summary	100%

General Project Information:

Parameter	Value
Type	New Flexible
Design Life	30 Years
Climate	C:\DG2002\Projects\bq1.icm
Construction Date	9/2010
Traffic Open Date	10/2010
Initial AADTT	1125

Properties

Setting	Value
Units	US Customary
Analysis Type	Probabilistic
Output Type	Excel Worksheet
Warnings	Enabled



Run Analysis

Traffic



Design Life (years):

50



Opening Date:

October, 2010

Initial two-way AADTT:

4000



Number of lanes in design direction:

2

Percent of trucks in design direction (%):

50.0

Percent of trucks in design lane (%):

72.4

Operational speed (mph):

62

Traffic Volume Adjustment:



Edit

Axle load distribution factor:



Edit

General Traffic Inputs



Edit



Import/Export

Traffic Growth

Compound, 2%



OK



Cancel

Traffic Volume Adjustment Factors



☒ Monthly Adjustment ☒ Vehicle Class Distribution ☒ Hourly Distribution ☒ Traffic Growth Factors

Load Monthly Adjustment Factors (MAF)

☐ Level 1: Site Specific - MAF

Load MAF From File

☒ Level 3: Default MAF

Export MAF to File

Monthly Adjustment Factors

Month	Class 4	Class 5	Class 6	Class 7	Class 8	Class 9	Class 10	Class 11	Class 12	Class 13
January	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
February	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
March	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
April	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
May	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
June	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
July	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
August	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
September	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
October	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
November	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
December	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

OK

Cancel

Traffic Volume Adjustment Factors



☒ Monthly Adjustment ☒ Vehicle Class Distribution ☒ Hourly Distribution ☒ Traffic Growth Factors

Hourly truck traffic distribution by period beginning:

Midnight	2.3	Noon	5.9
1:00 am	2.3	1:00 pm	5.9
2:00 am	2.3	2:00 pm	5.9
3:00 am	2.3	3:00 pm	5.9
4:00 am	2.3	4:00 pm	4.6
5:00 am	2.3	5:00 pm	4.6
6:00 am	5.0	6:00 pm	4.6
7:00 am	5.0	7:00 pm	4.6
8:00 am	5.0	8:00 pm	3.1
9:00 am	5.0	9:00 pm	3.1
10:00 am	5.9	10:00 pm	3.1
11:00 am	5.9	11:00 pm	3.1

Note: The hourly distribution must total 100%

Total:



OK



Cancel

General Traffic Inputs



Lateral Traffic Wander

Mean wheel location (inches from the lane marking):

18

Traffic wander standard deviation (in):

10

Design lane width (ft): (Note: This is not slab width)

12

☒ Number Axles/Truck

☒ Axle Configuration

☒ Wheelbase

Average axle width (edge-to-edge)
outside dimensions,ft):

8.5

Dual tire spacing (in):

12

Tire Pressure (psi)

120

Axle Spacing (in)

Tandem axle:

51.6

Tridem axle:

49.2

Quad axle:

49.2



OK



Cancel

Environment/Climatic



- ☒ Climatic data for a specific weather station.
☐ Interpolate climatic data for given location.

Latitude (degrees.minutes)
 Longitude (degrees.minutes)
 Elevation (ft)

☐ Seasonal

Depth of water table (ft)

Annual average

25

Note: Ground water table depth is a positive number measured from the pavement surface.

Select weather station

ANCHORAGE, AK
ANCHORAGE, AK
ANCHORAGE, AK
ANNETTE, AK
BARROW, AK
BETHEL, AK
BETTLES, AK
COLD BAY, AK
CORDOVA, AK
DEADHORSE, AK
DEERING, AK
DELTA JUNCTION/FT GREELY, A

Select Station

Cancel

Station Location:
(none)

Structure



Surface short-wave absorptivity:

Layers

Layer	Type	Material	Thicknes	Interface
1	Asphalt	Asphalt concrete	2.0	1
2	Asphalt	Asphalt concrete	7.0	1
3	Granular Base	Crushed stone	8.0	1
4	Granular Base	Crushed stone	30.0	1
5	Subgrade	CL	Semi-infini	n/a

Insert

Delete

Edit

Opening Date:

Design Life (years):

...



OK



Cancel

Asphalt Material Properties

Level: Asphalt material type:
Layer thickness (in):

☒ Asphalt Mix ☐ Asphalt Binder ☐ Asphalt General

Aggregate Gradation

Cumulative % Retained 3/4 inch sieve:
Cumulative % Retained 3/8 inch sieve:
Cumulative % Retained #4 sieve:
% Passing #200 sieve:

☒ OK ☐ Cancel ☒ View HMA Plots

Asphalt Material Properties

Level: Asphalt material type:
Layer thickness (in):

☒ Asphalt Mix ☐ Asphalt Binder ☐ Asphalt General

Dynamic Modulus Table

Number of temperatures: Number of frequencies:

Temperature (°F)	Mixture E* (psi)		
	0.1	1	10
10	2473184	3077411	3623623
70	851952	1401500	2016025
125	62656	73679	111244

☐ Import ☐ Export

☒ OK ☐ Cancel ☒ View HMA Plots

Asphalt Material Properties

Level: 3

Asphalt material type: Asphalt concrete

Layer thickness (in): 2

☒ Asphalt Mix ☒ Asphalt Binder ☒ Asphalt General

Options

☐ Superpave binder grading

☐ Conventional viscosity grade

☒ Conventional penetration grade

Pen Grade

☐ Pen 40-50

☐ Pen 60-70

☐ Pen 85-100

☐ Pen 120-150

☐ Pen 200-300

A VTS:

☒ OK ☒ Cancel

Asphalt Material Properties

Level: 1

Asphalt material type: Asphalt concrete

Layer thickness (in): 2

☒ Asphalt Mix ☒ Asphalt Binder ☒ Asphalt General

Options - At Short Term Aging - RTFO

☒ Superpave binder test data

☐ Conventional binder test data

Number of temperatures: 3

Temperature (°F)	Angular frequency = 10 rad/sec	
	G* (Pa)	Delta (°)
158	1067	65.55
169	675	67.09
172	521	69.04

☒ OK ☒ Cancel ☒ View HMA Plots

Unbound Layer - Layer #3

Unbound Material: Crushed stone Thickness(in): 8 ☐ Last layer

☒ Strength Properties ☒ ICM

Input Level

- ☐ Level 1:
☐ Level 2:
☒ Level 3:

Poisson's ratio: 0.35

Coefficient of lateral pressure, K_o : 0.5

Analysis Type

ICM Calculated Modulus

☒ ICM Inputs

User Input Modulus

- ☐ Seasonal input (design value)
☐ Representative value (design value)

Material Property

- ☒ Modulus (psi)
☐ CBR
☐ R - Value
☐ Layer Coefficient - a_1
☐ Penetration DCP (mr)
☐ Based upon PI and Gradation

AASHTO Classification

Unified Classification

Modulus (input) (psi): 16679

View Equation

Calculate >>

☒ OK

☐ Cancel

Avant de voir les résultats: Les équations

Distress Model Calibration Settings - Flexible New

AC Fatigue | AC Rutting | Thermal Fracture | CSM Fatigue | Subgrade Rutting | AC Cracking | CSM Cracking | IRI

$$N_f = 0.00432 * C * \beta_{f1} k_1 \left(\frac{1}{S_t} \right)^{k_2 \beta_{f1}} \left(\frac{1}{E} \right)^{k_3 \beta_{f1}}$$

$$C = 10^M$$

$$M = 4.84 \left(\frac{V_b}{V_a + V_b} - 0.69 \right)$$

- ☐ Special Analysis
- ☒ National Calibration
- ☐ State/Regional Calibration
- ☐ Typical Agency Values

k1: 0.007566

Bf1:

k2: 3.9492

Bf2:

k3: 1.281

Bf3:

Endurance limit for calculation of HMA Fatigue Damage

Avant de voir les résultats: Les équations

Unbound Aggregate Bases and Subbases

$$IRI = IRI_o + 0.0463 \left[SF \left(e^{\frac{age}{20}} - 1 \right) \right] + 0.00119(TC_L)_T + 0.1834(COV_{RD}) + 0.00384(FC)_T + 0.00736(BC)_T + 0.00115(LC_{SNWP})_{MH} \quad (3.3.74a)$$

where:

IRI	=	IRI at any given time, m/km.
IRI_o	=	Initial IRI, m/km.
SF	=	Site factor (see equation 3.3.74b).
$e^{\frac{age}{20}} - 1$	=	Age term (where <i>age</i> is expressed in years).
COV_{RD}	=	Coefficient of variation of the rut depths, percent (assumed to be 20 percent).
$(TC_L)_T$	=	Total length of transverse cracks (low, medium, and high severity levels), m/km).
$(FC)_T$	=	Fatigue cracking in wheel path, percent total lane area
$(BC)_T$	=	Area of block cracking as a percent of total lane area (user input).
$(LC_{SNWP})_{MH}$	=	Length of moderate and high severity sealed longitudinal cracks outside wheel path, m/km (user input).

$$SF = \left(\frac{(R_{SD})(P_{0.75} + 1)(PI)}{2 * 10^4} \right) + \left(\frac{\ln(FI + 1)(P_{0.2} + 1)[\ln(R_m + 1)]}{10} \right) \quad (3.3.74b)$$

where:

R_{sd}	=	Standard deviation of the monthly rainfall, mm.
$P_{0.75}$	=	Percent passing the 0.075 mm sieve.
PI	=	Percent plasticity index of the soil.
FI	=	Average annual freezing index, °C-days.
$P_{0.2}$	=	Percent passing the 0.02 mm sieve.
R_m	=	Average annual rainfall, mm.

Résultats

Project:

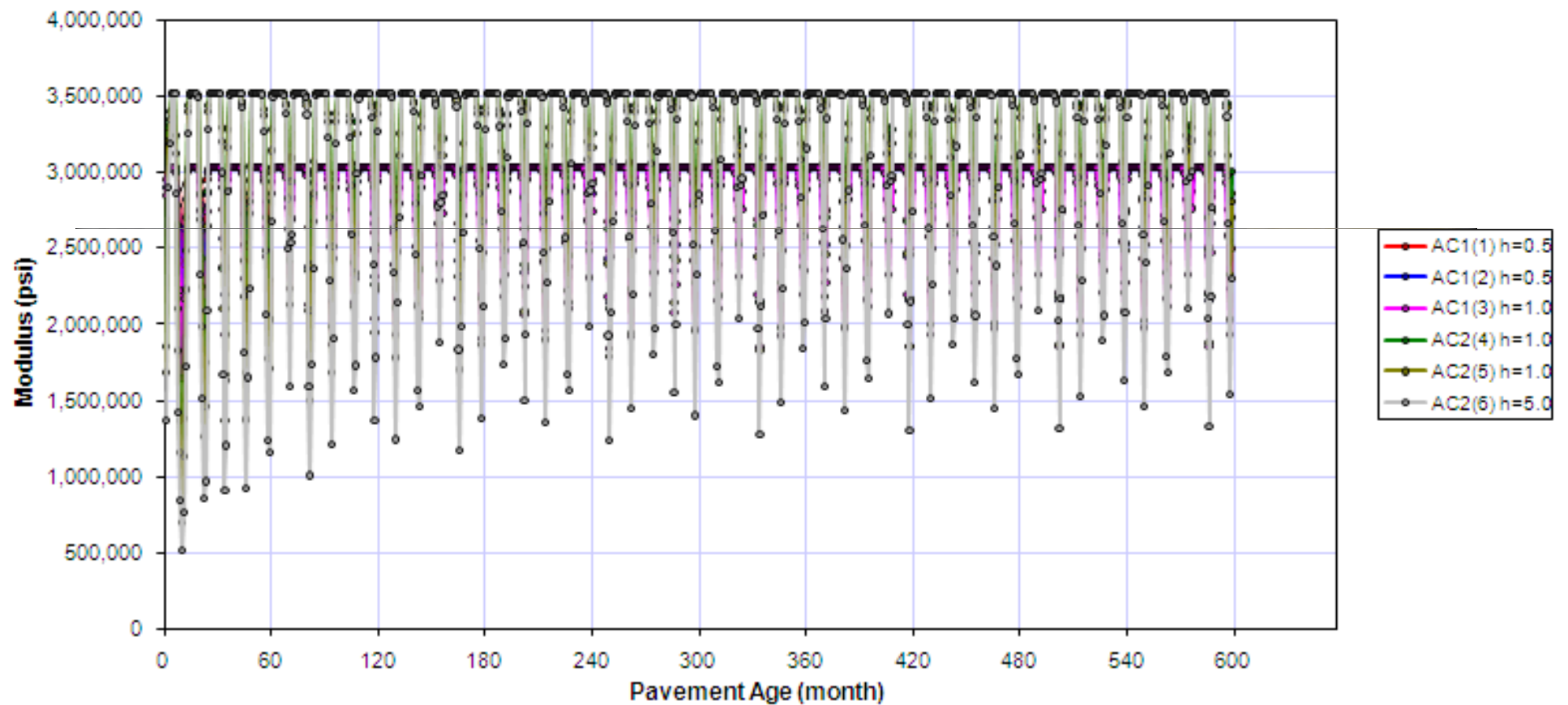
BQ_cas_1c_50ans_3

Reliability Summary

Performance Criteria	Distress Target	Reliability Target	Distress Predicted	Reliability Predicted	Acceptable
Terminal IRI (in/mi)	241	90	205	78.52	Fail
AC Surface Down Cracking (Long. Cracking) (ft/mile):	2000	90	0.1	99.999	Pass
AC Bottom Up Cracking (Alligator Cracking) (%):	25	90	1.3	99.999	Pass
AC Thermal Fracture (Transverse Cracking) (ft/mi):	1000	90	0	99.999	Pass
Chemically Stabilized Layer (Fatigue Fracture)	25	90			N/A
Permanent Deformation (AC Only) (in):	0.25	90	0.12	99.86	Pass
Permanent Deformation (Total Pavement) (in):	0.75	90	0.67	77.34	Fail

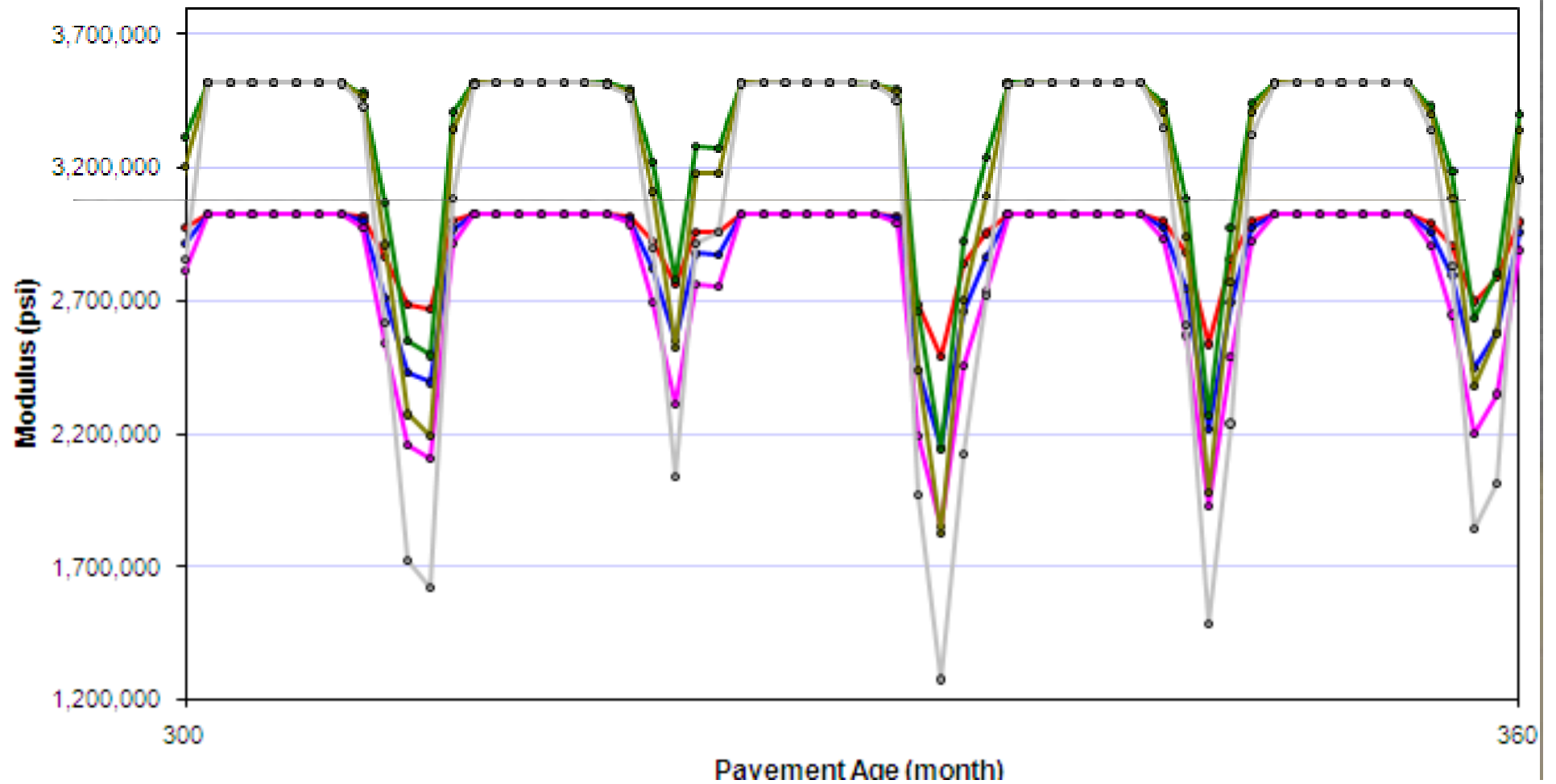
Résultats

Asphalt Sub-Layers Modulus Vs Time

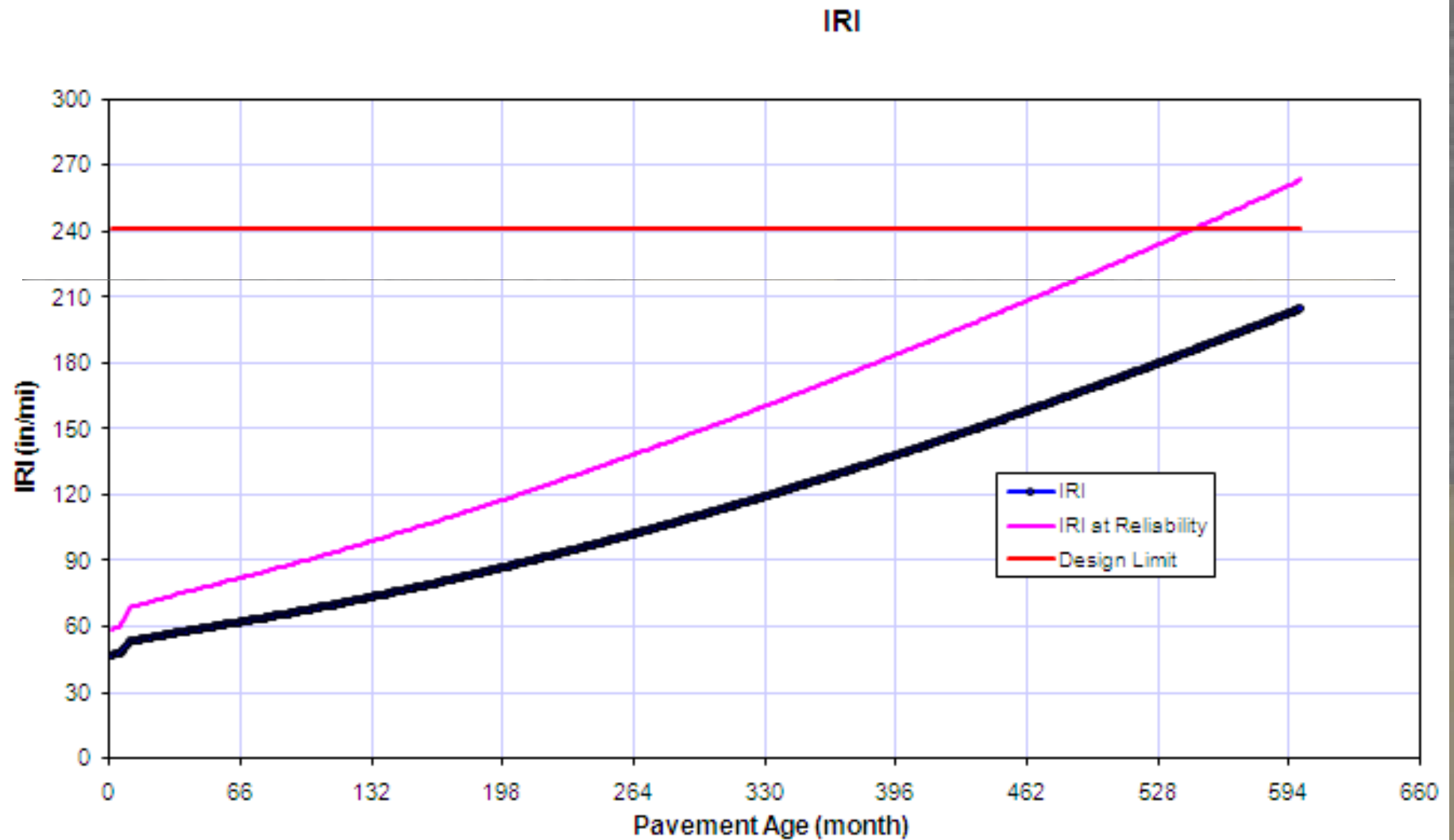


Résultats

Asphalt Sub-Layers Modulus Vs Time

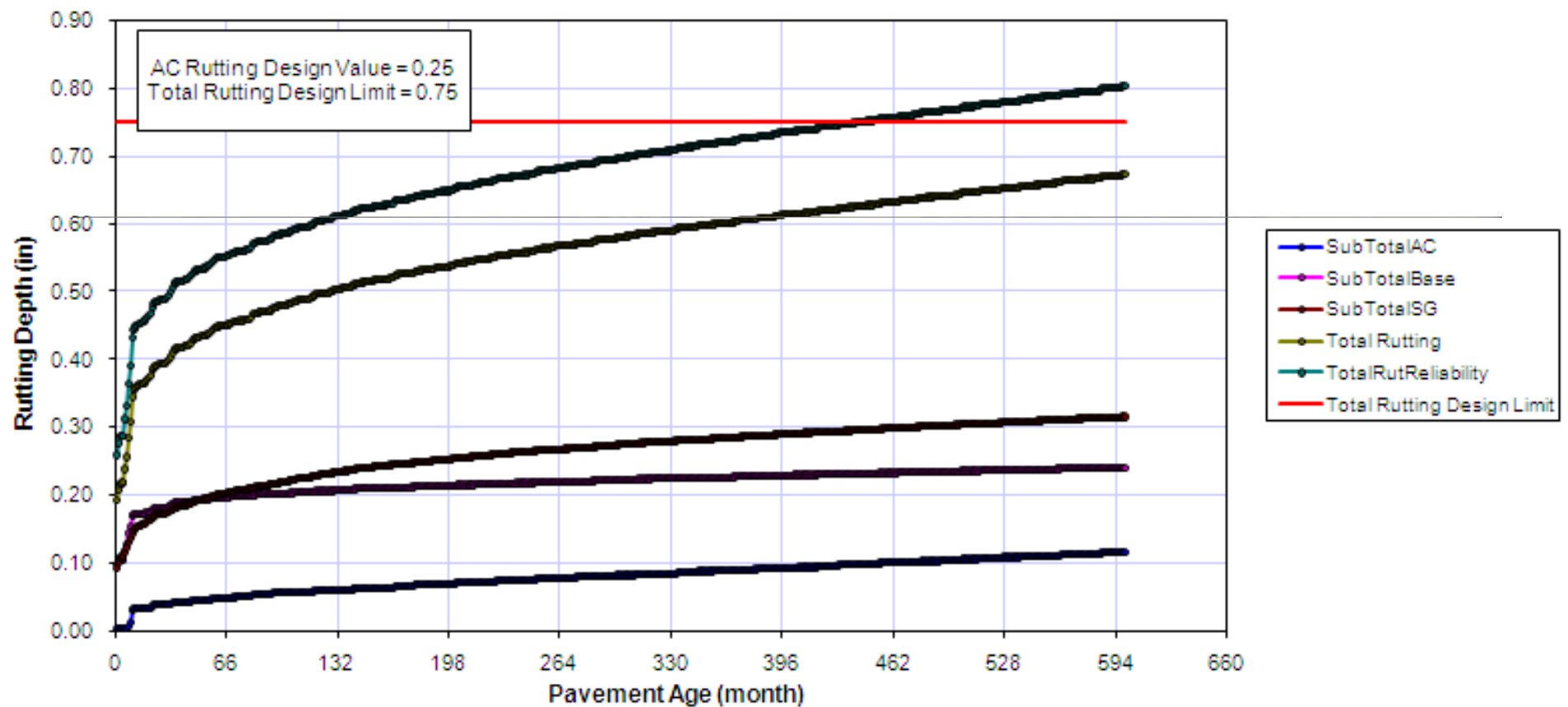


Résultats



Résultats

Permanent Deformation: Rutting



Résultats - autoroute



ESG-10 50mm

GB-20 175mm

MG-20 200mm

MG-112 750mm

IRI	45 ans
Fiss. (haut-bas)	> 50 ans
Fiss. (bas-haut)	>50 ans
Fiss. thermique	>50ans
Orni. enrobés	>50 ans
Orni. total	36 ans



ESG-10 100mm

MR-5 150mm

MG-20 200mm

MG-112 750mm

IRI	44 ans
Fiss. (haut-bas)	> 50 ans
Fiss. (bas-haut)	>50 ans
Fiss. thermique	>50ans
Orni. enrobés	>50 ans
Orni. total	25 ans

Résultats - urbain



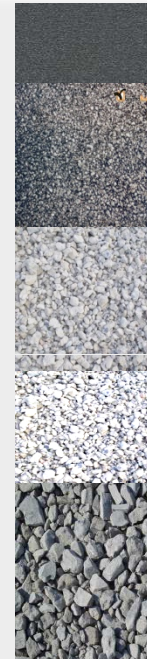
ESG-10 60mm

GB-20 110mm

MG-20 213mm

MG-112 575mm

IRI	>30 ans
Fiss. (haut-bas)	27 ans
Fiss. (bas-haut)	>30 ans
Fiss. thermique	>30ans
Orni. enrobés	>30 ans
Orni. total	25 ans



ESG-10 30mm

MR-5 traité
150mm

MR-5 150mm

MG-20 100mm

MG-112 450mm

IRI	44 ans
Fiss. (haut-bas)	> 50 ans
Fiss. (bas-haut)	>50 ans
Fiss. thermique	>50ans
Orni. enrobés	>50 ans
Orni. total	25 ans



Conclusions

- Méthode complète et complexe
 - Besoin d'intrants non disponible
 - Modèle de dégradation à revoir
 - Pas de dimensionnement au gel
-
- Il reste beaucoup de travail à faire



Merci de votre attention

